

108 學年度四技二專第三次聯合模擬考試 土木與建築群 專業科目(一) 詳解

108-3-06-4

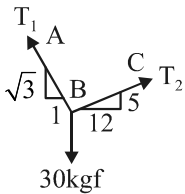
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
D	D	C	C	C	A	A	B	D	C	D	D	B	B	A	C	B	A	A	B
21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40
C	A	B	D	A	C	C	D	B	B	C	A	D	D	B	A	A	C	B	D

第一部分：工程力學

1. $G = \frac{E}{2(1+\mu)} = \frac{E}{2 \times 1.4} = \frac{5E}{14}$

$E_v = \frac{E}{3(1-2\mu)} = \frac{E}{3 \times (1-2 \times 0.4)} = \frac{5E}{3}$

2. 取 B 點：

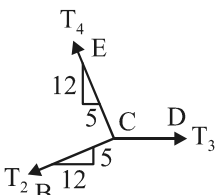


$\therefore T_1 = T_{AB}, T_2 = T_{BC}$

$$\begin{cases} \Sigma F_x = 0, \frac{T_1}{2} = \frac{12}{13} T_2 \dots\dots \textcircled{1} \\ \Sigma F_y = 0, \frac{\sqrt{3} T_1}{2} + \frac{5}{13} T_2 = 30 \dots\dots \textcircled{2} \end{cases}$$

得 $T_1 \doteq 27.92 \text{ kgf}$, $T_2 \doteq 15.13 \text{ kgf}$

取 C 點：

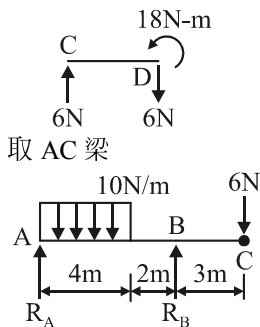


$\therefore T_3 = T_{CD}, T_4 = T_{CE}$

$$\begin{cases} \Sigma F_x = 0, \frac{12}{13} T_2 + \frac{5}{13} T_4 = T_3 \dots\dots \textcircled{3} \\ \Sigma F_y = 0, \frac{5}{13} T_2 = \frac{12}{13} T_4 \dots\dots \textcircled{4} \end{cases}$$

得 $T_3 \doteq 16.39 \text{ kgf}$, $T_4 \doteq 6.30 \text{ kgf}$

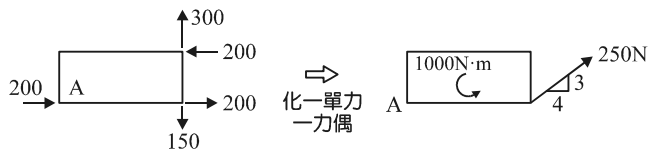
3. 取 CD 梁



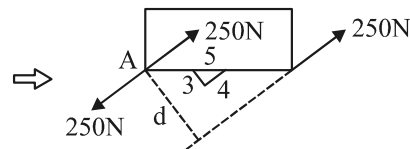
$(\curvearrowright) \Sigma M_A = 0$

$6R_B = 6 \times 9 + 10 \times 4 \times 2, R_B = 22.3 \text{ N} (\uparrow)$

4. 分解 250 N



$C_1 = 1000 \text{ N} \cdot \text{m} (\curvearrowleft)$

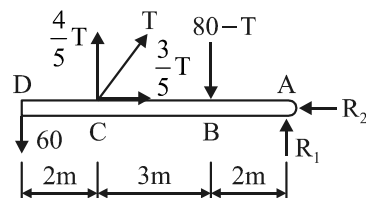


$\frac{d}{12} = \frac{3}{5}, d = 7.2 \text{ m}$

$\therefore P = 250 \text{ N}, C_2 = 1800 \text{ N} \cdot \text{m} (\curvearrowright)$

$C = C_1 + C_2 = 2800 \text{ N} \cdot \text{m} (\curvearrowright)$

5. 取梁之自由體圖



$(\rightarrow+) \Sigma F_x = 0 \Rightarrow R_2 = -58.02 \text{ kgf}$, 即 $58.02 \text{ kgf} (\leftarrow)$

$\Sigma M_A = 0$

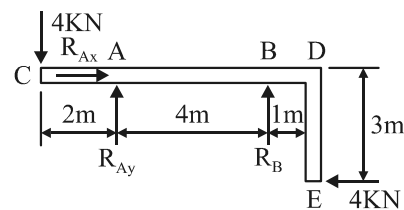
$60 \times 7 + (80 - T) \cdot 2 = \frac{4}{5} T \cdot 5, T = 96.7 \text{ kgf}$

$(\uparrow+) \Sigma F_y = 0$

$R_1 = 34.06 \text{ kgf} (\downarrow), R_2 = 58 \text{ kgf} (\leftarrow)$

$R_A = 67.3 \text{ kgf} (34.06 \frac{58}{67.3})$

6. 取自由體圖



$(\curvearrowright) \Sigma M_A = 0$

$4 \times 3 = 4 \times 2 + 4R_B, R_B = 1 \text{ kN} (\uparrow)$

$$\begin{aligned} \Sigma F_y = 0, R_{Ay} &= 3 \text{ KN } (\uparrow) \\ \Sigma F_x = 0, R_{Ax} &= 4 \text{ KN } (\rightarrow) \\ \therefore R_A &= 5 \text{ KN } (\nearrow) \end{aligned}$$

7. 分解 $F_1 = 130 \text{ kgf}$

$$\begin{cases} F_{1x} = 130 \times \frac{4}{\sqrt{4^2 + 3^2 + 12^2}} = 40 \text{ kgf } (\leftarrow) \\ F_{1y} = 130 \times \frac{12}{13} = 120 \text{ kgf } (\uparrow) \\ F_{1z} = 130 \times \frac{3}{13} = 30 \text{ kgf } (\swarrow) \end{cases}$$

分解 $F_2 = 110 \text{ kgf}$

$$\begin{cases} F_{2x} = 110 \times \frac{6}{\sqrt{6^2 + 6^2 + 7^2}} = 60 \text{ kgf } (\rightarrow) \\ F_{2y} = 110 \times \frac{7}{11} = 70 \text{ kgf } (\downarrow) \\ F_{2z} = 110 \times \frac{6}{11} = 60 \text{ kgf } (\nearrow) \end{cases}$$

$$\begin{cases} \Sigma F_x = 20 \text{ kgf } (\rightarrow) \\ \Sigma F_y = 50 \text{ kgf } (\uparrow) \\ \Sigma F_z = 30 \text{ kgf } (\nearrow) \end{cases}$$

$$R = \sqrt{20^2 + 50^2 + (-30)^2} = 61.6 \text{ kgf}$$

8. $M_{ab} = 40 \times 9 + 30 \times 2 - 60 \times 6$

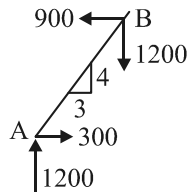
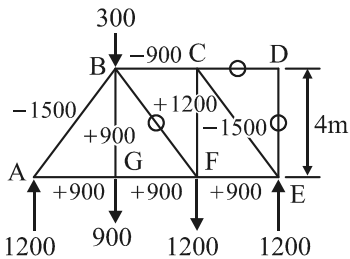
$$= 360 + 60 - 360 = 60 \text{ kgf-m } (\uparrow)$$

9. 求反力

$$R_A = 1200 \text{ kgf } (\uparrow), R_E = 1200 \text{ kgf } (\uparrow)$$

取節點 A 如右

$$\therefore S_{AB} = 1500 \text{ kgf } (\text{壓力})$$



零桿件為 3 支

$$S_{CD} = S_{BF} = S_{DE} = 0 \text{ kgf}$$

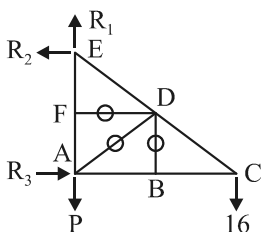
拉力桿件: $S_{AG}, S_{GF}, S_{BG}, S_{CF}, S_{EF}$

壓力桿件: S_{AB}, S_{BC}, S_{CE}

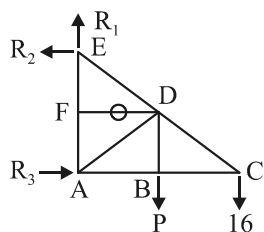
10. $(\curvearrowright) \Sigma M_E = 0$

$$16 \times 8 = 6 \cdot R_A, R_A = 21.33 \text{ KN}$$

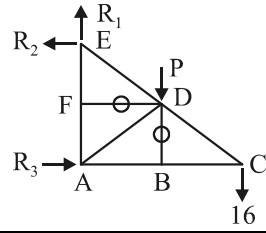
在 A 點作用 P 力, 3 支零桿



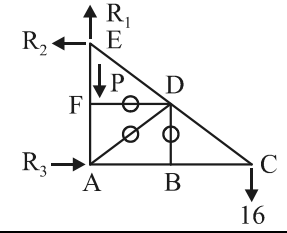
在 B 點作用 P 力, 1 支零桿



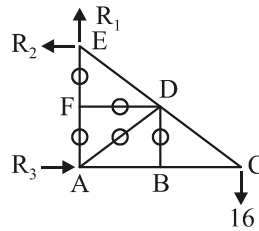
在 D 點作用 P 力, 2 支零桿



在 F 點作用 P 力, 3 支零桿



原桁架零桿 5 支, 故在 A 點或 F 點作用 P 力均會減少 2 支零桿

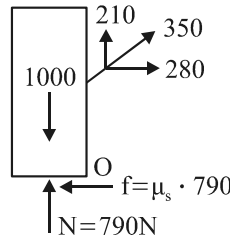


11. $(\rightarrow) \Sigma F_x = 0, 790\mu_s = 280, \mu_s = 0.35$

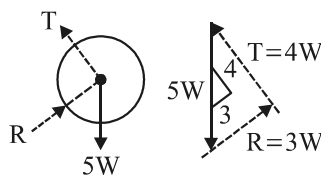
所以當 μ_s 小於 0.35 即產生運動

ΣM_O 時, 傾覆力矩 $M = 280 \times 2.5 = 700$

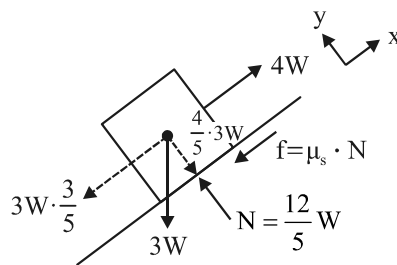
穩定力矩 $M = 1000 \times 1 = 1000$, 故不傾倒



12. 取球體之自由體



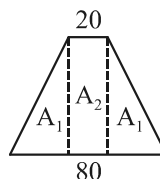
取方塊物體自由體



$$(\nearrow) \Sigma F_x = 0, \frac{9}{5}W + \mu_s \cdot \frac{12}{5}W = 4W$$

$$9 + 12\mu_s = 20, \mu_s = \frac{11}{12}$$

13. 將圖倒立計算



$$A_1 = \frac{1}{2} \cdot 30 \cdot 60 = 900 \text{ cm}^2$$

$$A_2 = 20 \cdot 60 = 1200 \text{ cm}^2$$

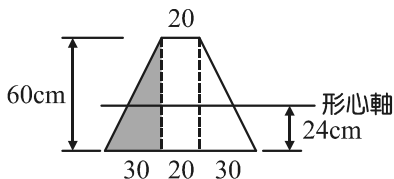
$$y = \frac{2 \cdot A_1 \cdot 20 + A_2 \cdot 30}{2A_1 + A_2} = \frac{2 \cdot 900 \cdot 20 + 1200 \cdot 30}{2 \cdot 900 + 1200} = 24 \text{ cm}$$

∴ 形心軸至底邊之距離為 60 - 24 = 36 cm

14. 形心慣性矩 I_G ，利用平行軸定理

$$I_G = 2 \cdot \left[\frac{30 \times 60^3}{36} + 900 \cdot 4^2 \right] + \frac{20 \times 60^3}{12} + 1200 \cdot (30 - 24)^2$$

$$= 388800 + 403200 = 792000 \text{ cm}^4$$

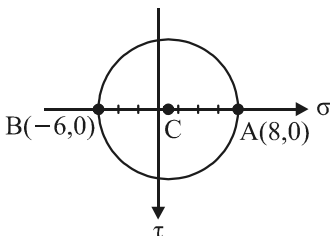


15. 軸向應變 $\epsilon = \frac{P}{AE} = \frac{500}{20 \times 50000} = 5 \times 10^{-4}$

16. $\epsilon = \frac{\delta}{l}$
 ∴ $\delta = \epsilon \cdot l = 0.002 \times 80 = 0.16 \text{ cm}$
 $\epsilon_v = \frac{\sigma(1-2\mu)}{E} = 0.002 \cdot (1-2 \cdot 0.3)$
 $= 0.002 \times 0.4 = 8 \times 10^{-4}$

17. 由莫耳圓來看

A(8, 0), B(-6, 0), 圓心(1, 0)



$$\tau = \text{半徑} = \frac{AB}{2} = \frac{\sqrt{(8-(-6))^2 + 0^2}}{2} = \frac{\sqrt{14^2}}{2} = 7$$

$$\sigma_1 = \text{圓心} + r = 1 + 7 = 8 \text{ N/cm}^2$$

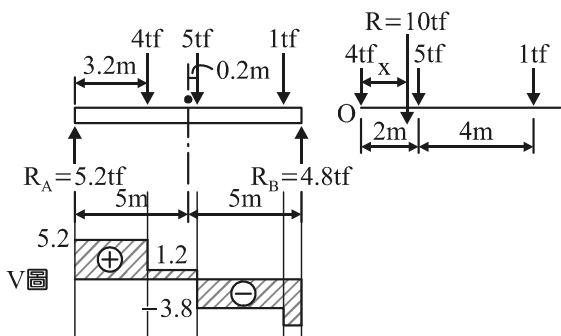
$$\sigma_2 = 1 - 7 = -6 \text{ N/cm}^2$$

18. E、G、 E_v 三者關係

$$G = \frac{E}{2(1+\nu)}, E_v = \frac{E}{3(1-2\nu)}, E = \frac{9E_v G}{G+3E_v}$$

$$E > E_v > G$$

19.



$$\Sigma M_O = 0, 5 \cdot 2 + 1 \cdot 6 = 10 \cdot x, x = 1.6$$

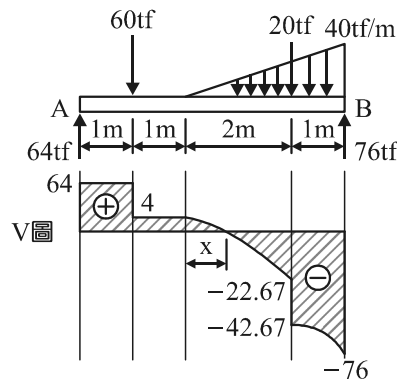
$$10 \times 4.8 - R_B \times 10 = 0, R_B = 4.8 \text{ tf} (\uparrow)$$

$$10R_A = 4 \times 6.8 + 5 \times 4.8 + 1 \times 0.8$$

$$10R_A = 52, R_A = 5.2 \text{ tf} (\uparrow)$$

$$M_{\max} = 4.8 \times 4.8 - 1 \times 4 = 19.04 \text{ tf-m}$$

20.



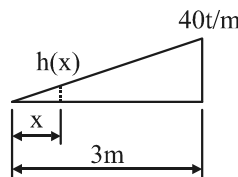
$$(\curvearrowright) \Sigma M_B = 0$$

$$R_A = \frac{(60 \times 4 + 20 \times 1 + \frac{1}{2} \times 40 \times 3 \times 1)}{5} = 64$$

$$R_A = 64 \text{ tf} (\uparrow)$$

$$(\uparrow) \Sigma F_y = 0, R_B = 76 \text{ tf} (\uparrow)$$

計算 V = 0 之位置



$$\frac{3}{40} = \frac{x}{h(x)}, h(x) = \frac{40x}{3}$$

$$\therefore V = 0 \text{ 時}, 4 = \frac{1}{2} \cdot x \cdot \frac{40x}{3}, \frac{5x^2}{3} = 1$$

$$x^2 = \frac{3}{5}, x = \sqrt{\frac{3}{5}} = 0.77$$

$$M_{\max} = 64 \times 1 + 4 \times 1 + \frac{2}{3} \cdot 0.77 \cdot 4 = 70.05 \text{ tf-m}$$

V = 0 處距 A 點 2.77 m

第二部分：工程材料

21. (A) 鬆比重為含孔隙及水之材料與同體積 4°C 水之重量比
- (B) 比熱為使 1 公克重之材料溫度升高 1°C 所需之熱量
- (D) 虎克定律為 $\sigma = E \cdot \epsilon$
22. (A) 華格納氏濁度計在求水泥之細度
23. (B) C₂S 的水化速度慢，水化熱低
24. (D) 抗彎試體為 4×4×16 cm 長方體，試驗台兩反力支撐點距離定為 10 cm
25. (A) 7 天之抗壓強度約為 28 天抗壓強度之 $\frac{2}{3}$
26. (C) 新拌混凝土之體積變化有塑性收縮、浮水、凝結

收縮

27. (C) 減水劑不會降低強度，可減少用水量約 10%
28. 細粒料的標準篩包含#4、#8、#16、#30、#50、#100
29. (B) 輝長岩為火成岩
30. (B) 三種磚之吸水率應低於 15%
31. (C) 強化玻璃製法為利用熱處理將玻璃加熱到臨近軟化點溫度，使急速冷卻增加強度
34. (D) $3(2 \times 1.5 \times 0.4) + 2(2 \times 1 \times 0.6) = 3.6 + 2.4 = 6$ 才
35. (B) 角材之材積，1 才 = 1 寸 × 1 寸 × 10 尺

角材 材積	1 才 = 1 台寸 × 1 台寸 × 1 丈 = 1 台寸 × 1 台寸 × 10 台尺 = 100 立方台寸
板材 材積	1 才 = 1 台尺 × 1 台尺 × 1 台寸 = 1 台寸 × 10 台寸 × 10 台寸 = 100 立方台寸 (惟板材厚度若不足 1 台寸時需另加鋸路損耗 1 台分)

36. 常用之安定劑為各種鉛及金屬、肥皂、鉛鹽、有機錫化合物等
37. (A) 鐵氟龍(聚四氟乙烯 PTFE)
38. 丙、金屬比重大於 4 以上稱重金屬
戊、鋼的韌性在熱處理的回火
39. (B) SD280W 屬於竹節鋼筋，標稱直徑為 15.9 mm 以上
40. (D) 在結構上之橋梁、鋼軌使用中碳鋼